Durable Continuously Reinforced Concrete Pavements through Active Crack Control



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Characteristics Continuous Reinforced Concrete Pavements (CRCP)

Crack pattern measurements (spacing, crack width) on 3 motorways:

- E17 near Ghent (B), no crack control
- A50 near Eindhoven (NL), no crack control
- E313 near Herentals (B), active crack control

Concluding remarks



- Longitudinal reinforcement 0.7%-0.75%
- Reinforcement mid-depth or higher
- Control shrinkage crack pattern (spacing, width)
- Criteria for reinforcement: crack width < 0.4 mm (NL,B)
- No transverse joints
- Increasingly applied in Europe (especially Belgium) and USA on heavily loaded motorways





Pavement structure (reconstruction E17 in Belgium in 2011), dimensions in cm



reinforcement asphalt interlayer



Longitudinal reinforcement (e.g. ø 20 mm @ 180 mm)



Minimum percentage of longitudinal reinforcement to prevent yield of rebars

Concrete grade	C25/30	C30/37	C35/45	C45/55
ω0,min	0.38	0.43	0.47	0.54

Practical percentage of longitudinal reinforcement (in North-west Europe)

Concrete grade	C25/30	C30/37	C35/45	C45/55
ω0	0.7	0.7	0.7	0.75



CRCP investigations

Investigations into crack pattern: - E17 near Ghent, Belgium: . reconstruction in August 2011 . no active crack control - E313 near Herentals, Belgium: . reconstruction in September 2012 . active crack control - A50 near Eindhoven, the Netherlands: . 8 years old, 70 mm twinlayer PA wearing course . no active crack control . reflective cracking in PA wearing course



3 test sections

Test section	Chainage (km)	Longitudinal reinforcement
1	44.7 – 45.2	0.75%
2	45.2 – 46.2	0.70%
3	46.2 – 46.7	0.65% + 20 kg/m ³ steel fibers

Crack pattern (spacing and width) measurements:

- regularly during 4 days and nights after construction in August 2011
- 5 times in period October 2011 April 2014

Crack width measurements at pavement surface





Change of crack width measurements ad pavement edge (LVDT measurements on glued studs across crack)



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Typical cracks





Y-cracks

Cluster of closely spaced cracks





Development of crack pattern



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Development of crack spacing









Development of crack width at pavement surface in section 1

Age (days)	Temperature at pavement	Nu	mber		Crack w	idth (mm)	
	surface (°C)	cracks	readings	mean	maximum	minimum	standard deviation
4	30.3	8	40	0.169	0.22	0.10	0.043
223	8.8	14	42	0.117	0.30	0.03	0.076



Crack width changes at pavement edge in test section 1, 3 days after construction

Depth	Temperature			Cra	ck numb	er			mean
pavement surface (mm)	change (°C)	1	2	3	4	5	6	7	
0	22.0-30.2	0.132	0.202	0.328	0.182	0.190	0.170	0.206	0.201
30		0.136	0.212	0.222	0.195	0.154	0.127	0.158	0.172
90*		0.131	0.178	0.192	0.161	0.133	0.084	0.102	0.140

* Centre of longitudinal reinforcement



Risk of closely spaced cracks: punchouts





Early 2015 1 punchout observed in wheel track of test section 3 (0.65% + 20 kg/m³ steel fibers)



70 mm twinlayer PA wearing course on CRCP

8 years after construction: reflective cracking







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100 m section, air temperature 5°C (November 2013), pavement age 8 years, 0.67% longitudinal reinforcement

Crack pattern:

- average crack spacing 1.92 m (large!)
- 27% of crack spacings \leq 0.6 m (a lot)
- 41% of crack spacings 0.6 2.4 m
- 32% of crack spacings \geq 2.4 m (a lot!)
- 15 out of 52 cracks (30%) reflected through PA



Cores taken on 9 cracks for crack width measurements

Core	Crack width (mm)			Sum of crack spacings at	Reflective
number	top	middle	bottom	both sides of the crack (m)	crack
1 2 3 4 5 6 7	0.35 0.23 0.29 0.55 0.35 0.18 0.20	0.25 0.30 0.10 0.30 0.30 0.18 0.17	0.35 - 0.25 0.41 0.30 0.20 0.29 0.24	5.90 4.25 3.85 4.00 4.00 3.15 2.45 2.25	Yes No Yes Yes No No
8 9	- 0.32	0.28	0.34	-	yes
average	0.31	0.24	0.31	-	-

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Cores taken on 9 cracks for crack width measurements



To prevent reflective cracks in this specific case: - crack width < 0.25 mm - sum of crack spacings < 3.2 m, i.e. crack spacings < 1.6 m

Mid 2014 (age 9 years) PA milled off and replaced by SAMI plus 70 mm twinlayer PA

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Reconstruction in 2012 2-lift concrete pavement (50 mm + 200 mm = 250 mm) Longitudinal reinforcement 0.75%, 90 mm below pavement surface 50 mm asphalt interlayer

Trial to control crack pattern through surface notches (short transverse saw cuts), applied within 16 hours after construction

Top view





Geometry of notches





Cracks at notches

Depth Section		Age No. of (days) notches		o. of No. of No. of No. of cracks		Effecti-	Percentage of cracks in category (%)			
cut (mm)	(m)	(augo)	(N1)	(N2)	at notches	notches N3/N1	Distance to	nearest	notch (m)	
					(N3)	(%)	0 (N3/N2)	0-0.2	0.2-0.4	0.4-0.6
60	1100	1 2 3 4 65 204* 378 555**	897	1 73 163 193 664 762 775 803	1 71 161 191 555 597 606 628	0.1 7.9 17.9 21.3 61.9 66.6 67.6 70.0	100 97.3 98.8 99.0 83.6 78.3 78.2 78.2	0 0 0 2.4 3.8 3.8 3.6	0 0 0 7.7 9.8 9.9 10.1	0 2.7 1.2 1.0 6.3 8.1 8.1 8.1
30	500	123 262* 613**	422	417 497 505	245 281 286	58.1 66.6 67.8	58.8 56.5 56.6	9.4 8.7 8.7	15.8 17.5 17.2	16.0 17.3 17.5

* after 1st winter

** after 2nd winter

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Straight cracks





crack at notch

crack in between 2 notches





Crack spacing frequency distributions





Comparison crack pattern on E17 and E313

Crack spacing after about 20 months (including 2 winters) on E17 and E313, 0.75% reinf.



mean crack spacing

crack spacing frequency distribution

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Comparison crack pattern on E17 and E313

Crack spacing after about 20 months (including 2 winters) on E17 and E313, 0.75% reinforcement

Motorway	Test section	Crack sp	Crack spacing distribution (%)					
		≤ 0.6 m	0.6 – 2.4 m	≥ 2.4 m	crack spacing (m)			
E17	1	51.8	29.4	19.8	1.18			
E313	60 mm notches 30 mm notches	14.0 29.6	75.0 65.9	11.0 4.5	1.35 0.99			



Comparison crack pattern on E17 and E313

Crack width changes due to temperature on E17 and E313, 0.75% reinforcement

Motor-	Test	Tempe-	Number	Crack width (mm)				
way	section	rature of pave- ment surface (°C)	of cracks	mean	maximum	minimum	standard deviation	
E17	1	30.3	8	0.169	0.22	0.10	0.036	
		2.2	10	0.312	0.35	0.19	0.087	
E313	60 mm	20.5	17	0.152	0.31	0.10	0.032	
	notches	8.0	12	0.201	0.27	0.14	0.034	
	30 mm notches	21.0	11	0.198	0.22	0.13	0.035	
		4.2	11	0.232	0.32	0.13	0.044	



Concluding remarks

Active crack control through partial surface notches seems quite successful:

- Less closely spaced cracks and less widely spaced cracks
- So more crack spacings in preferred range 0.6 2.4 m
- Very straight cracks (not interfering)
- Cheap measure

But:

- Only 3 years of experience
- Time of saw cutting partial surface notches very important (current practice: within 16 hours of concrete placement)











Muito obrigado



Types of concrete pavements

1. Jointed plain concrete pavements (JPCP):

- Through joints divided into slabs, 5 * 4 m
- Dowel and tie bars in transverse and longitudinal joints, resp.
- Joints mostly sealed
- No reinforcement at all
- Most widely applied
- Empirical and analytical design methods



Empirical design method: RStO 11 (Ger)

RStO 11 (Germany)

Only JPCP

type of base

					(Dickenang	gaben in cm; 🗕	▪ E _{v2} -Mindest	werte in MN/m²)
Zeile	Bauklasse	7	6	5	4	3	2	1
	В	> 32	> 10 - 32	> 3,2 - 10	> 1,8 - 3,2	> 1,0 - 1,8	> 0,3 - 1,0	≤ 0,3
	Dicke des frostsich.Oberbaues ¹⁾	55 65 75 85	55 65 75 85	55 65 75 85	45 55 65 75	45 55 65 75	45 55 65 75	35 45 55 65
	Vliesstoff auf Tragschic	ht mit hydraulis	chem Bindemit	<u>tel</u> auf Frostsch	utzschicht bzw	-		
	Schicht aus irostunemp		1000 1000 1000 1000 1000 1000 1000 100	22		1 122	I	1
	Betondecke	27	25	25	24	23		
11	Vilesstoff */	15	15	15	15	15		
	Tragschicht (HGT)	<u>* 120</u>	<u>- 120</u> Σ41	- 120 Σ40	- 120 Σ 30	= 120 		
	Frostschutzschicht	<u>45 0.0</u>	<u>45 0.0</u>	<u>45 0.0</u>	<u>450.0</u>	<u>45 0.0</u>		
	Dicke der Frostschutzschicht	<u>33</u> 2 43	- 24 ³⁾ 34 44	- 253 35 45	263 36	273) 37		
	Betondecke	72	22	222	22	22		
	Viiesstofi 8)	27	26	25	24	23		
1.2	Verfestigung	20	15	15	15	15		
	Schicht aus frostunempfindlichem Material		5.0 Σ41	0. o Σ 40	O.° o Σ39	0°0'Σ38		
	-welt- oder intermittierend gestuft gemäß DIN 18196-	<u>45 0.0</u> Σ 47	<u>• 45 0 000</u>	<u>45000</u>	<u>45 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0</u>	<u>45 0000</u>		
	Dicke der Schicht aus trostunempfindlichem Material	84) 184) 28 38	144) 24 34 44	154 25 35 45	64) 16 26 36	273 37		
	Betondecke	7/2 -	77 20	7/ 25	20.00	Zan	20	20
	Viiesstoff 8)	4	- <u>-</u>	<u> </u>	<u> </u>		15	11
1.3	Verfestigung	25	20	20	20	20	10	13
	Schicht aus Frostungemfindlichem Material		0 0°Σ 46	0 ° ° Σ 45	ο ο ο Σ 44	0 0 Σ 43	0 0 2 33	000
	-enggestuft gemäß DIN 18196-	<u>- 45 0.00</u> 2.00	- 45 (0,09	<u>- 45 0.09</u>	<u>- 45 000</u>	<u>- 45 000</u>	<u>- 45 (0,09</u>	<u>- 45 (0,09</u>
\vdash	frostunempfindlichem Material	34) 134) 23 33	10* 19 29 39	104 20 30 40	14) 114) 21 31	24) 124 22 32	10+ 20 30 40	- 10 20 30
	Asphalttragschicht auf	Frostschutzschi	cht FZ/1	8771	1 8721	1 1727	ı	1
	Betondecke	26	25	24	23	22		
2	Asphalttracschicht	4m XX 10	120 10	120 10	120 10	<u>120 8</u>		
-		- 120 × × · ° ° Σ35	0.0 Σ35	0.0°Σ 34	Ο, Ο, Σ 33	C • Ο Σ 30		
	Frostschutzschicht	- 45 0.0	- 45 0.0	_ 45	<u>45</u>	- 45		
	Dicke der Frostschutzschicht	- 293) 39 49	- 30 ²⁾ 40 50	- 31 ² 41 51	322 42	- 253) 35 45	- 212 31 41	- 21 ²⁾ 31 41
	Schottertragschicht auf	Schicht aus fro	stunempfindlic	hem Material				
	Betondecke	29	28	27	26	24		
_		- <u>150</u>	- 150 91.10	- 150 PLIO	- 150 75 P	- <u>150</u>		
3	Schotlertragschicht	¢ 25	0 0 25	0 0 25 0 0				
	Schicht aus	- 45 C · O Σ 54	<u>45 Q.0</u> Σ53	<u>45</u> <u>2</u> 52	<u>_ 45 Q 0</u> Σ 51	<u>45</u> <u>2</u> 49		
	Dicke der Schicht aus	At	12 cm aus frostunen	npfindlichem Material.	geringere Restdicke is	st mit dem darüber lied	enden Material ausz	deichen
	Frostschutzschicht				5		,	
	Betondecke						- 100 21	100 21
							0°0 Σ 21	δ
4	Frostschutzschicht						0.0	0.0
							45 0.0	45 0.0
	Dicke der Ernstschutzschicht						243 24 44 54	143 24 24 44
	Linke der Prosischulzschicht						24~134 44 54	144 24 34 44
1) Be Ma	e abweichenden Werten sind die D sterials durch Differenzbildung zu b	icken der Frostschulzs estimmen, siehe auch	chicht bzw. des frostu Tabelle 8	nempfindlichen	 Nur auszuführen, Material als eine 5 	wenn das frostunemp Schicht eingehaut wen	findliche Material und	das zu verfestigende

traffic loading

2) Mit rundkörnigen Gesteinskörnungen nur bei örtlicher Bewährung anwendbar

3) Nur mit gebrochenen Gesteinskömungen und bei örtlicher Bewährung anwendbar

8) Anstelle des Vilesstoffes kann eine Asphaltzwischenschicht gewählt werden

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Analytical design method: VENCON2.0 (NL)





Types of concrete pavements

2. Pre-stressed concrete pavements:

- Large slabs, 130 * 30 m
- Pre-stressed in 2 directions
- Thin pavement (airport: 180 mm)
- Very complex joints
- Most expensive type of pavement
- Few applications (Amsterdam Airport Schiphol)



Types of concrete pavements

Amsterdam Airport Schiphol (55 million passengers/year)





CRCP on A5 near Amsterdam (no crack control) 50 mm single layer PA wearing course on CRCP 13 years after construction: reflective cracking



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Three 100 m sections, air temperature 2°C-5°C (February 2015), pavement age 13 years, 0.60% longitudinal reinforcement

Crack pattern:

- average crack spacing 4.41 m (very large!!)
- 5% of crack spacings \leq 0.8 m
- 29% of crack spacings 0.8 3.0 m (very few!)
- 66% of crack spacings \geq 3.0 m (an awful lot!)
- 39 out of 68 cracks (57%!) reflected through PA



Crack width characteristics at pavement surface (37 cracks)

Crack width parameter	With reflective cracks	Without reflective cracks
Number of cracks	24	13
Mean crack width (mm)	0.45	0.38
Standard deviation of crack width (mm)	0.14	0.10
Maximum crack width (mm)	0.91	0.59
Minimum crack width (mm)	0.27	0.23



Crack width characteristics at pavement surface (37 cracks)





Cores taken on 12 cracks for crack width measurements

Crack width parameter	With reflective cracks	Without reflective cracks
Number of cracks	2	6
Mean crack width (mm)	0.448	0.371
Standard deviation of crack width (mm)	0.057	0.059
Maximum crack width (mm)	0.488	0.468
Minimum crack width (mm)	0.408	0.295



Cores taken on 12 cracks for crack width measurements



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70 mm twinlayer PA wearing course on CRCP

8 years after construction: few reflective cracks







On Eastern carriageway, 5 sections, total 1500 m, air temperature 10°C—>25°C (April 2015), pavement age 8 years, 0.70% longitudinal reinforcement mid-depth

Crack pattern very good:

- average crack spacing 1.88 m
- 18% of crack spacings ≤ 0.8 m
- 67% of crack spacings 0.8 3.0 m
- 15% of crack spacings ≥ 3.0 m
- 15 out of 803 cracks (only 2%!) reflected through PA

On Western carriageway, 6 sections, total 1062 m, air temperature 9°C—>17°C (April 2015), pavement age 8 years, 0.70% longitudinal reinforcement mid-depth

Crack pattern less good:

- average crack spacing 2.63 m (a bit large)
- 11% of crack spacings \leq 0.8 m
- 57% of crack spacings 0.8 3.0 m
- 32% of crack spacings \geq 3.0 m
- 27 out of 404 cracks (7 2%) reflected through PA



Crack width measurements not successful (filled with dust, a bit spalling)

A73 quite good performance:

- crack pattern (near to) optimal
- double tack coat between CRCP and PA
- polymer modified bitumen applied in both top layer and bottom layer of PA (normally only in top layer)
- few reflective cracks, no other damage
- no PA replacement needed in near future

Comparison crack spacings

Comparison of crack spacing frequency distributions: huge differences!



